

EVALUATION OF SELF-COMPACTING RECYCLED CONCRETE ROBUSTNESS BY STATISTICAL APPROACH

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Abstract

The use of self-compacting recycled concrete appears as to be a very interesting technology for the sustainable construction future. However, one of the major obstacles to a more widespread use of self-compacting concrete is to obtain a robust material. Therefore, the emphasis of this work is placed on analysing both practice and theory to understand the properties that control and assess self-compacting recycled concrete robustness.

Hence, forty-nine different mixes were produced with several replacement percentages of recycled concrete coarse aggregate (0, 20, 50 or 100%) and with two different mixing procedures (all aggregates in dry-state conditions or recycled aggregate with a 3% of natural moisture). The experimental program consisted of making, in the fresh state, rheological tests (a stress growth

33 test and a flow curve test) and [empirical characterization tests](#) (slump flow, V-funnel, L-box, J-Ring
34 and sieve segregation) at 15, 45 and 90 min from cement-water contact. [In the hardened state](#),
35 compressive strength was measured at 3, 7 and 28 days.

36 All results were [analysed using](#) a statistical approach based on Kendall's coefficient of concordance
37 and Spearman's rank correlation. This approach allowed us to successfully identify six key
38 properties that can be measured to evaluate SCRC robustness ([capacity of the material to tolerate](#)
39 [certain variations in material characteristics and mixture parameters](#)). For each mix, a ranking that
40 [defines its robustness category was obtained by considering all properties](#). Also, it showed that
41 water control is the key factor that affects SCRC robustness.

42

43 **Keywords:** self-compacting concrete; recycled aggregate; mixing procedure; robustness; rheology;
44 statistical approach.

45

46 **1 INTRODUCTION AND OBJECTIVES**

47 In the near future, using recycled materials [in conventional and high performance applications](#)
48 should be a priority area [1]. At this stage, it is fundamental to analyse the characteristics of
49 recycled materials, recycling procedures and manufacturing processes. The main difference
50 between natural aggregate and the recycled concrete aggregate is the adhered mortar [2, 3]. The
51 presence of this material decreases with the number of crushing processes, the size fraction and
52 the original waste quality [4, 5].

53 In general terms, the quality of vibrated recycled concrete is lower than that of conventional
54 concrete with the same mix proportions [6, 7]. Many of the current studies in vibrated recycled
55 concrete field deal with short-term analysis related to basic properties and structural

56 performance, and a few of them have studied the long-term behaviour [8, 9]. The compressive and
57 splitting tensile strengths and modulus of elasticity decrease when the percentage of recycled
58 aggregate increases, and the shrinkage and creep increase deformations [10, 11]. These variations
59 are mostly due to the adhered mortar.

60 On the other hand, self-compacting concrete is a highly flowable concrete that spreads rapidly into
61 place and fills formwork without vibrating compaction in order to ease casting and to achieve
62 durable concrete structures [12, 13]. At the construction site, it has increasingly been used over
63 the past two decades and it is empirically described according to its filling ability, passing ability
64 and segregation resistance [14]. Most of studies state that, if a SCC is well designed, it can provide
65 similar mechanical properties to its equivalent vibrated concrete [15]. However, the SCC flow
66 properties and its fresh rheological behaviour diverge from what is expected from vibrated
67 concrete of normal consistency [16].

68 One of the major obstacles to a more widespread use of self-compacting concrete is to obtain a
69 robust material [17, 18]. Robustness is the capacity of a concrete to maintain its properties when
70 changes in materials, mixing parameters or environmental variables take place [19, 20].

71 Self-compacting concrete has shown to be more sensitive to variations in its design process than
72 vibrated concrete [21, 22]. The mix design is a critical step to obtain high quality self-compacting
73 concrete. A large number of variables must be considered in the mix design process and its
74 interactions are difficult to predict [23].

75 Different studies have been developed to analyse self-compacting concrete robustness. In general,
76 aggregate density and size, paste density, type of mixer, mixing protocol, mixing time and total
77 mixing energy are factors that have to be taken into account to analyse robustness [24]. Some
78 works conclude that robustness can be influenced by the water to powder volume ratio, the
79 superplasticiser to powder weight ratio and the solid volume [25-27]. Others state that errors in

80 weighing water and fines content [19] or those affecting aggregate moisture [28] are of capital
81 importance.

82 Lastly, a new material, self-compacting recycled concrete (SCRC) appears as a self-compacting
83 concrete made with recycled aggregate, in this work, recycled concrete coarse aggregate. This
84 concrete has to combine successfully the behaviour of a self-compacting concrete and that of a
85 vibrated recycled concrete [29]. The materials used to produce SCRC are the same as in self-
86 compacting concrete, but recycled aggregates are used as replacement of natural aggregates [30,
87 31]. The type and shape of coarse aggregate, combined gradation of sand and coarse aggregate,
88 content of cement and supplementary cementitious materials, paste volume, and water to
89 powder ratio must be considered when designing SCRC as in self-compacting concrete [32-35]. The
90 use of recycled aggregate could improve the environmental aspects of self-compacting concrete
91 without significant impact on workability and strength characteristics when low replacement
92 percentages are used (up to 50%) [36-39]. However, not so much works have studied the
93 rheological properties of SCRC, measuring the static yield stress and plastic viscosity [30, 38, 40,
94 41], and analysed the specificity of its rheological behaviour [42].

95 Keeping the above in mind, extensive scientific research has been developed on vibrated recycled
96 concrete over the last decades [7, 11]. At the same time, high performance concretes have
97 become a great challenge and one of the most remarkable topics in the field of materials
98 engineering. In this context, the use of self-compacting concrete introducing new variables, as the
99 replacement of natural aggregates with recycled aggregates, appears as to be a very interesting
100 technology for the sustainable construction future.

101 As a consequence, SCRC has been studied only for a short time and there is a significant gap in the
102 knowledge of its robustness [43]. SCRC involves multi-physics phenomena related to the specific
103 intrinsic characteristics of recycled aggregates and the other components and variables of

104 concrete design. Therefore, the emphasis of this work is placed on analysing both practice and
105 theory to understand the properties that control and assess SCRC robustness.

106 In order to be successful in this approach, a statistical analysis is made with results from a wide
107 experimental program. Taking into account the work of Naji et al. [21] on conventional self-
108 compacting concrete, Kendall's coefficient of concordance and Spearman's rank correlation can be
109 used to evaluate self-compacting recycled concrete robustness and to select adequate concrete
110 properties that could be measured to determine it. Therefore, in this work, a statistical approach
111 to SCRC robustness is carried out with the aim of determining which tests provide more sensitivity
112 when the robustness of a SCRC mix is evaluated.

113 **2 METHODOLOGY**

114 Two research stages were conducted, an experimental stage and an analytical stage. The former
115 consisted of 49 mixes of SCRA in which several replacement percentages of recycled aggregate and
116 relevant parameters (mixing procedure and constituent materials) were varied. In the second
117 stage, a statistical approach was performed to draw general conclusions and to reduce the
118 number of properties that could provide a reliable understanding of SCRC robustness.

119 **2.1 Testing program**

120 In this work, the studied mixes were prepared with a Portland cement (CEM-I 52.5-R), with a
121 density of 3110 kg/m³ and a specific surface (BET) of 1.02 m²/g. A limestone filler was also used
122 with a density of 2710 kg/m³ and a specific surface (BET) of 1.77 m²/g. The properties of cement
123 and filler are given in Table 1 and Table 2. A superplasticiser (a modified polycarboxylate) was used
124 as chemical additive. It showed a solid content of 35% and a density of 1080 kg/m³. This kind of
125 superplasticiser is used to produce high performance, high strength and flowable concretes.

126

Table 1. Properties of cement

CEM-I 52.5-R	
Physical and mechanical properties	
Initial setting time	190 min
Final setting time	260 min
Soundness	0.3 mm
Initial strength	45.5 MPa
Strength	64 MPa

127

128

Table 2. Chemical composition of cement and filler

Oxide/Element	CEM-I 52.5-R (%)	Filler (%)
CaO	64.1	54.7
SiO ₂	15.9	1.6
SO ₃	4.3	0.18
Al ₂ O ₃	4.1	0.46
Fe ₂ O ₃	4.0	0.22
K ₂ O	1.3	0.12
MgO	1.1	0.47
SrO	0.78	0.046
Na ₂ O	0.27	-
TiO ₂	0.25	-
ZnO	0.12	0.009
Cl	0.059	-
P ₂ O ₅	0.050	-
MnO	0.047	-
CuO	0.040	0.010
ZrO ₂	0.036	0.003
PbO	0.022	-
Loss on ignition (1000 °C)	3.2	41.8

129

130 The fine aggregate was a crushed limestone sand with a nominal size of 0-4 mm, a fineness
 131 modulus of 4.19, a saturated-surface-dry density of 2720 kg/m³ and a water absorption capacity of
 132 1%. As coarse aggregates, a crushed granitic natural aggregate and a recycled fraction obtained
 133 from real demolition debris of structural concrete were used, both with a nominal size of 4-11
 134 mm. The natural coarse aggregate showed a fineness modulus of 7.14, a saturated-surface-dry
 135 density of 2560 kg/m³ and a water absorption capacity of 1.12%.

136 The recycled coarse aggregate was made up mainly of concrete and stone. So, it was a recycled
 137 concrete coarse aggregate. Its fineness modulus was 6.47 and the main properties are presented

138 in Table 3. It is remarkable that after 10 min it absorbs up to 80% of its total water absorption at
 139 24 hours. This percentage was taken into account when all recycled concretes were produced.

140 **Table 3. Main physical properties and composition of recycled aggregate**

Particle size (mm)	Physical properties			Composition (%)			
	ρ_{ssd} (kg/m ³)	Absorption 24 h (%)	Absorption 10 min (%)	Natural aggregate and aggregate with mortar	Ceramic	Asphalt	Rest
4/11	2340	6.96	5.57	96.35	0.79	0.48	3.25

141

142 The design of mixes consisted of a reference mix and three recycled mixes with 20%, 50% and
 143 100% replacement percentages of recycled coarse aggregate (by volume) (Table 4). Two mixing
 144 procedures were also used, one using aggregates in dry-state conditions (M1 method) and another
 145 where the recycled aggregate was used with a 3% of natural moisture (M3 method). Therefore,
 146 seven baseline mixes were designed (SCRC0, SCRC20M1, SCRC50M1, SCRC100M1, SCRC20M3,
 147 SCRC50M3, SCRC100M3).

148 **Table 4. Mix proportions of reference concrete (1 m³)**

SCRC0 – Dosage	
Cement, c (kg)	400
Filler, f (kg)	180
Water, w (kg)	184
Natural sand (kg)	866
Natural coarse aggregate (kg)	768
Effective w/c	0.46
Superplasticiser/(c+f) (%)	0.6

149

150 Moreover, the study of robustness of mixes produced with M1 and M3 method (SCRC0,
 151 SCRC20M1, SCRC50M1, SCRC100M1, SCRC20M3, SCRC50M3, SCRC100M3) has been made using
 152 water variations (W+, 0, W-, that corresponds to +3%, base, -3%) and superplasticiser variations
 153 (S+, 0, S-, that corresponds to +5%, base, -5%). Robustness of mixes produced with M1 method
 154 (SCRC0, SCRC20M1, SCRC50M1, SCRC100M1) was also studied using cement variations (C+, 0, C-
 155 that corresponds to +3%, base, -3%).

156 Recycled concretes were produced by adding an extra quantity of water during mixing. This was
157 calculated to compensate the 80% of recycled aggregate absorption at 24 h. The mixing protocol
158 for both M1 and M3 methods was as follows: firstly, the aggregates (sand and coarse aggregates)
159 were mixed with the extra water for 2 min and then left to rest for another 8 min; secondly, the
160 cement was added along with the filler. After 2.5 min of mixing, water was added (98.5%). This
161 cement-water contact is considered the reference time for performing all fresh concrete tests.
162 After 2 min of mixing, the superplasticiser and the remaining water were introduced. The mixing
163 was continued for another 3 min, the concrete was left to rest for 2 min and finally mixed again for
164 an additional 2 min.

165 Regarding tests methods, on the one hand, rheology was studied throughout two tests: a stress
166 growth test and a flow curve test. The parameters measured with these tests were the static yield
167 stress (τ_0) and the plastic viscosity (μ_{pl}) respectively.

168 A rotational portable rheometer with a four-bladed vane was used to conduct the rheological
169 tests. Firstly, the stress growth test was made at a low and constant speed of 0.025 rps as soon as
170 the vane of the rheometer was immersed into the concrete. After that, the vane was removed, the
171 concrete remixed, the vane reinserted and the flow curve test started. After a breakdown period
172 of 20 s at a constant speed of 0.5 rps, the torques at decreasing speeds were measured in seven
173 steps. In this research, according to previous works [29], the Bingham model was applied to the
174 five data points obtained with the lowest rotational speeds in the flow curve test.

175 On the other hand, workability was studied with several empirical characterization tests: slump
176 flow (EN 12350-8 [44]), V-funnel (EN 12350-9 [45]), L-box (EN 12350-10 [46]), J-Ring (EN 12350-12
177 [47]) and sieve segregation (EN 12350-11 [48]). The parameters measured with these tests were:
178 slump flow diameter (SF), time of 500 mm slump flow (t500), time of V-funnel (tv), blocking
179 coefficient (PL), J-Ring diameter (SFJ), time of J-Ring (t500J), blocking step (PJ) and sieve
180 segregation percentage (SR).

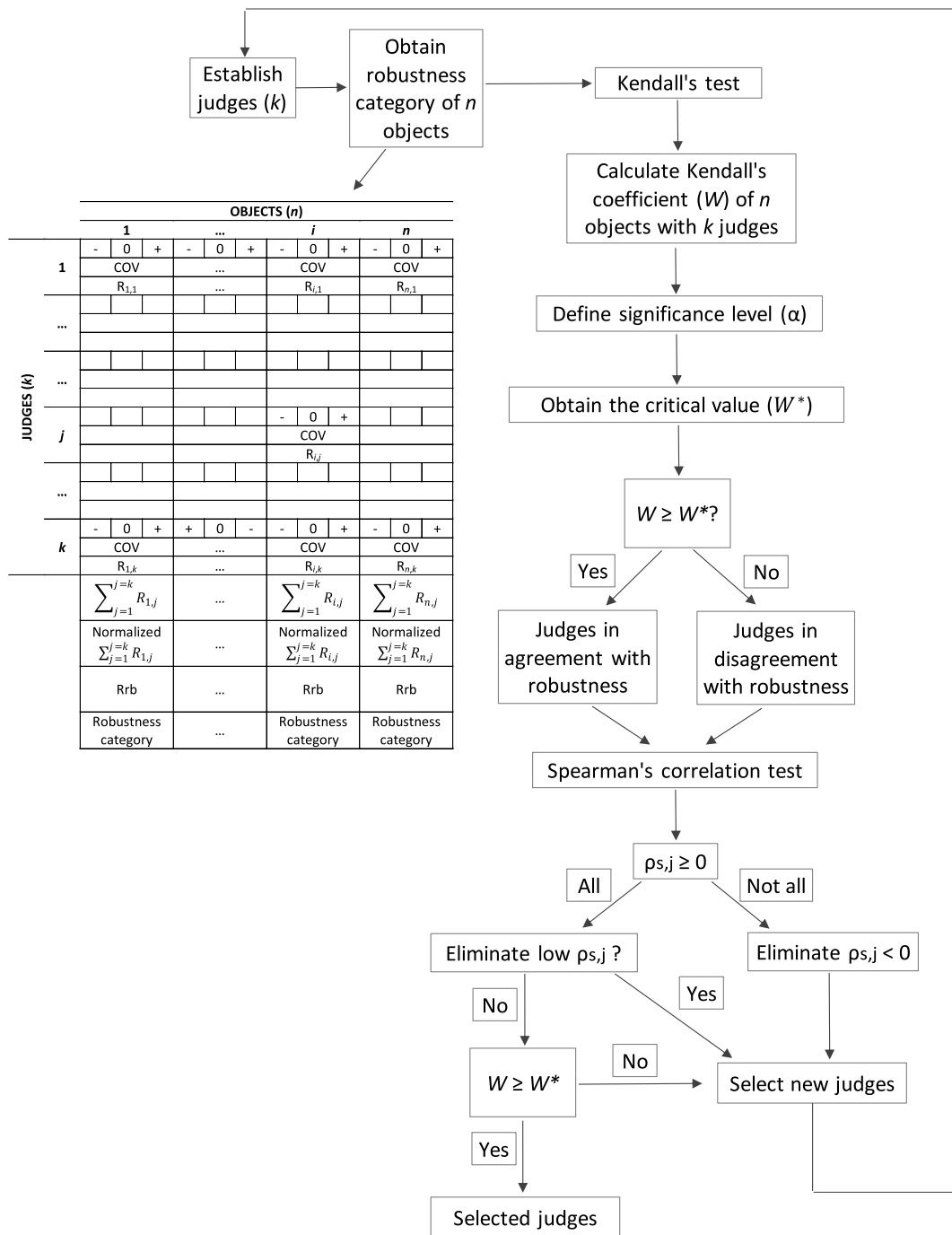
181 Rheological and [empirical characterization tests](#) were made over time (at 15, 45 and 90 min from
182 cement-water contact) and all obtained results were used for developing the statistical approach.
183 Also, results of compressive strength (f_c) at different ages (3, 7 and 28 days) were incorporated
184 into the statistical analysis.

185 **2.2 Analytical investigation**

186 Kendall's coefficient of concordance is a measure of the agreement among several k "judges" used
187 to assess a characteristic of a given set of n objects. The method is used to evaluate the degree of
188 agreement among several "judges" [49]. The methodology used in this work is summarized in
189 Figure 1.

190 In this study, n (the objects to be assessed) are the different mixes characterised by their recycled
191 aggregate percentage (0, 20, 50 or 100%) and the mixing method (M1 or M3). Therefore, when
192 water and superplasticiser variations are imposed, M1 and M3 methods are used and then $n = 7$.
193 However, when cement variations are analysed, only M1 method is used, then, in this case, $n = 4$.

194 Each object (i object, with i from 1 to n) is going to be ranked using different "judges" as assessors
195 or a single judge applying different criteria. Then, a rank $R_{i,j}$, with i from 1 to n and with j from 1 to
196 k , is obtained in each object for each judge based on the coefficients of variation obtained with
197 each judge.



198

199

Figure 1. Flow chart of statistical approach methodology

200

In this work, when water and superplasticiser variations are imposed, 31 properties were

201

considered as the “judges of robustness” ($k = 31$) and the coefficients of variation (COVs) obtained

202

with each judge were used to rank the seven mixes ($n=7$). In the case of cement variations, 26

203

properties were considered ($k = 26$) to rank the four mixes ($n = 4$). Each COV is obtained for each

204 object (mix) and for each judge (property) with the results of the baseline mix (“0”) and of the
 205 same mix with the two material variations (increase, “+”, and decrease, “-”). Therefore, these
 206 COVs are used to rank each object (mix) within each judge (property), $R_{i,j}$.

207 The result of the judgment (concrete robustness) can be obtained summing, in each object (mix),
 208 the ranks ($R_{i,j}$) gotten with each judge (property) (Eq. 1).

$$209 \quad SR_i = \sum_{j=1}^{j=k} R_{i,j} \quad i = 1 \cdots n \quad (1)$$

210

211 This result (SR_i) can be normalized and then used to define SCRC robustness. This “normalized sum
 212 of ranking” (0-100%) (Eq. 2) will be used to rank the objects according to their robustness, “Rrb”
 213 (from more robust to less robust). Moreover, this can be used to define a category (high, medium,
 214 low) that classifies the robustness of each SCRC mix [21].

$$215 \quad \text{Normalized sum of ranking (\%)} = \frac{(SR_{max} - SR_i)}{(SR_{max} - SR_{min})} 100 \quad (2)$$

216 Being:

$$217 \quad SR_{max} = \max(SR_i) \quad i = 1 \cdots n \quad (3)$$

$$218 \quad SR_{min} = \min(SR_i) \quad i = 1 \cdots n \quad (4)$$

219

220 On the left of the Figure 1, a flow chart is shown to summarize this part of the methodology.

221 Once the characteristic (robustness) has been assessed, it is necessary to be sure that there is
 222 agreement among the “judges” used. To check this, the significance of Kendall’s coefficient has to
 223 be evaluated.

224 For this purpose, the Kendall’s coefficient (W) is calculated for the sample. To evaluate its
 225 significance, a significance level (α) is chosen and then the critical value of W (W^*) is calculated for

226 this significance level. If the observed W is greater than or equal to the critical value W^* , then the
 227 null hypothesis (there is no agreement among the “judges”) may be rejected at that level of
 228 significance, i.e. the “judges” are in agreement (there is concordance among them) in the
 229 assessment of the characteristic (robustness).

230 Therefore, firstly, the Kendall’s coefficient is calculated as follows:

$$231 \quad W = \frac{S}{\frac{1}{12} \cdot k^2 \cdot (n^3 - n)} \quad (5)$$

232 Being:

$$233 \quad S = \sum_{i=1}^n (SR_i - SR)^2 \quad (6)$$

$$234 \quad SR = \frac{(n+1) \cdot k}{2} \quad (7)$$

235

236 Then, to test whether an observed value of W is significant, it is necessary to consider the
 237 distribution of W . The actual distribution of W is irregular for low values of k and n , and likely to be
 238 quite irregular for moderate values [49].

239 Regarding small samples, the distribution of W under H_0 (null hypothesis, the assumption that the
 240 judges are in disagreement) has been worked out and the critical values of Kendall’s coefficient
 241 (W^*) can be obtained taking into account the approximation based on Fisher’s z -distribution with
 242 ν_1 and ν_2 degrees of freedom (Eqs. 8-10). The “ z ” values have been tabled for the following
 243 different significance levels, $\alpha = 0.05$ and $\alpha = 0.01$ [50].

$$244 \quad z = \frac{1}{2} \log_e \frac{(k-1)W}{1-W} \quad (8)$$

$$245 \quad \nu_1 = n - 1 - \frac{2}{k} \quad (9)$$

$$246 \quad \nu_2 = (k - 1)\nu_1 \quad (10)$$

247

248 For large samples, Friedman’s test can be used to determine the significance of W . The Friedman’s
 249 test statistic is distributed approximately as chi-square (χ^2), with $(n - 1)$ degrees of freedom (Eq.
 250 11). In this case, also, for a desired level of significance and a particular value of n , under the null
 251 hypothesis (H_0), the critical values (W^*) can be obtained.

$$252 \quad \chi^2 = k(n - 1)W \quad (11)$$

253

254 When W equals or exceeds the critical value W^* obtained for a desired level of significance, the
 255 null hypothesis (the assumption that the judges are in disagreement) may be rejected. That is, the
 256 k “judges” (properties) are in agreement with each other and it can be concluded that there is a
 257 good consensus among them concerning the evaluation of the characteristic (robustness) of the n
 258 objects (mixes).

259 On the right of the Figure 1, the flow chart shows this part of methodology.

260 Lastly, when the significance of Kendall’s coefficient was evaluated, the correlation between the
 261 rankings of an individual “judge” ($R_{i,j}$) and the final ranks of the objects, “ Rrb ”, has to be assessed.

262 To do so, Spearman’s correlation test can be used.

263 Spearman’s correlation test calculates the Spearman’s rank correlation coefficient or Spearman’s
 264 ρ_s . It is a non-parametric measure of statistical correlation between two ranked variables [51], and
 265 it can be expressed as follows:

$$266 \quad \rho_{s,j} = 1 - \frac{6 \cdot \sum_{i=1}^n (R_{i,j} - Rrb_i)^2}{n \cdot (n^2 - 1)} \quad (12)$$

267

268 Spearman’s $\rho_{s,j}$ ranges between -1 and 1 and measures the correlation between rankings obtained
 269 with an individual judge ($R_{i,j}$) and the final ranks of the objects, “ Rrb ”. A positive value of $\rho_{s,j}$ implies

270 a positive correlation among the two series of rankings. On the contrary, a negative $\rho_{s,j}$ value
271 indicates a no correlation between them.

272 Therefore, the result of this test allows us to eliminate those judges which provide no correlation
273 and/or those which provide a low correlation. In this way, the number of judges may be reduced,
274 simplifying the characteristic assessment. In any case, if the number of judges is changed, it is
275 necessary to check that Kendall's coefficient maintains a value higher than the critical one
276 according to the desired level of significance. Once this has been done, it can be concluded that
277 the selection of judges that provide the best correlation to assess the characteristic is achieved.

278 **3 RESULTS AND DISCUSSION**

279 **3.1 Robustness category**

280 In the study of robustness of mixes produced with M1 and M3 method using water and
281 superplasticiser variations, thirty-one properties of SCRC were used as "judges". These properties
282 include six rheological properties, three mechanical ones and twenty-two workability parameters.
283 Therefore, seven mixes ($n = 7$, SCRC0, SCRC20M1, SCRC50M1, SCRC100M1, SCRC20M3,
284 SCRC50M3, SCRC100M3) were analysed with 31 properties ($k=31$).

285 In the case of robustness of mixes produced with M1 method using cement variations, twenty-six
286 properties were used as "judges". These properties include six rheological properties, three
287 mechanical ones and seventeen workability parameters. Therefore, four mixes ($n = 4$, SCRC0,
288 SCRC20M1, SCRC50M1, SCRC100M1) were analysed with 26 properties ($k=26$).

289 Table 16, Table 17 and Table 18 (see Appendix) present the rheological, mechanical and
290 workability properties obtained in mixes where water, superplasticiser and cement variations
291 were imposed, respectively. The COV values obtained with each property and the corresponding
292 ranking assigned to each mix are also presented. If a property value does not appear on the tables,

293 this means that it was not possible to develop the test to measure it due to the loss of self-
294 compactability. Then, this mix was ranked with the highest ranking value.

295 In the three cases (see Appendix: water variations, Table 16, superplasticiser variations, Table 17,
296 and cement variations, Table 18), the COVs obtained with each property were calculated for each
297 mix. Based on the COV values, the SCRC mixes were ranked. The mix with the lowest COV value is
298 the mix that presents the best level of robustness, so this mix will be ranked with the number “1”
299 and so on.

300 At this step, all properties are considered to evaluate robustness and then, for each mix, all the
301 individual rankings have been summarized obtaining a “SR_i” value. This has been used to rank the
302 mixes (within each material variation) according to their robustness, “Rrb” (from more robust to
303 less robust). Moreover, the sum of rankings SR_i has been normalized according to Eq. 2. Table 16,
304 Table 17 and Table 18 (see Appendix) also show all these values for water, superplasticiser and
305 cement variations, respectively.

306 Finally, according to the normalized sum of ranking, a category (high, medium-high, medium-low,
307 low) that classifies the robustness has been selected (Table 5).

308

Table 5. SCRC robustness classification

Normalized sum of ranking	Robustness category
> 90%	High
60% to 90%	Medium-High
30% to 60%	Medium-Low
≤ 30%	Low

309

310 Then, Table 6 and Table 7 summarize the robustness category of the investigated mixes obtained
311 with each of the three different material variations (water, superplasticiser and cement).

312 As seen in Table 6, when water and superplasticiser variations are analysed, the 20% replacement
313 concretes (SCRC20M1 and SCRC20M3) show a medium-high level of robustness and SCRCs with a

314 50% of recycled aggregate display medium-high and medium-low robustness for M1 and M3
 315 methods, respectively. Regarding the 100% replacement concretes, the M1 method provides a
 316 SCRC mix with a medium-low or low robustness whereas the M3 method always provide a
 317 concrete with a normalized sum of ranking $\leq 30\%$, which is considered as a low level of robustness.
 318 This mix will be, then, the least robust.

319 **Table 6. Evaluation of SCRC robustness (water and superplasticiser variations)**

Mix	Water variations		Superplasticiser variations	
	Normalized sum of ranking (%)	Robustness	Normalized sum of ranking (%)	Robustness
SCRC0	100	High	100	High
SCRC20M1	70	Medium-High	73	Medium-High
SCRC50M1	63	Medium-High	67	Medium-High
SCRC100M1	28	Low	40	Medium-Low
SCRC20M3	86	Medium-High	87	Medium-High
SCRC50M3	58	Medium-Low	62	Medium-High
SCRC100M3	0	Low	0	Low

320

321 When cement variations are observed (Table 7), these robustness categories are corroborated in
 322 general terms. As seen, the 20% replacement concrete shows a high level of robustness, the
 323 SCRC50M1 mix displays medium-low robustness and the 100% replacement percentage provides a
 324 concrete with a low robustness.

325 **Table 7. Evaluation of SCRC robustness (cement variations)**

Mix	Cement variations	
	Normalized sum of ranking (%)	Robustness
SCRC0	100	High
SCRC20M1	96	High
SCRC50M1	56	Medium-Low
SCRC100M1	0	Low

326

327 **3.2 Selection of SCRC properties to evaluate robustness**

328 According to methodology, once the characteristic (robustness) has been assessed, it is necessary
 329 to be sure that there is agreement among the “judges” (properties) used. To check this, the

330 Kendall's coefficient has to be calculated and its significance measured. Table 8, Table 9 and Table
 331 10 show the Kendall's coefficient of concordance among concrete properties that were used as
 332 "judges" for water, superplasticiser and cement variations respectively.

333 **Table 8. Kendall's coefficient and Spearman's $\rho_{s,j}$ (water variations)**

SCRC	τ_0 (15')	μ_{pl} (15')	τ_0 (45')	μ_{pl} (45')	τ_0 (90')	μ_{pl} (90')	$f_{c,3d}$	$f_{c,7d}$	$f_{c,28d}$	t500 (15')	SF (15')	tv (15')	PL (15')	t500j (15')	SFJ (15')	PJ (15')	SR	t500 (45')	SF (45')	tv (45')	PL (45')	t500j (45')	SFJ (45')	PJ (45')	t500 (90')	SF (90')	tv (90')	PL (90')	t500j (90')	SFJ (90')	PJ (90')	Rrb
	$R_{i,j}$																															
0	1	1	1	1	1	2	5	4	2	1	1	2	1	3	2	2	1	2	1	3	2	4	1	4	1	1	1	2	1	2	1	
20M1	2	2	3	2	3	3	6	6	7	2	3	5	2	2	4	6	2	5	4	5	3	3	2	3	5	3	2	4	2	2	1	3
50M1	3	5	4	4	2	5	2	1	5	4	6	4	4	4	3	3	4	4	2	4	4	5	4	2	4	5	4	3	4	4	4	4
100M1	6	6	6	6	6	6	4	5	1	5	4	3	6	6	6	5	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6	6
20M3	4	4	2	3	5	4	1	3	3	3	2	1	3	1	1	1	3	1	3	1	1	1	3	5	2	4	5	1	3	3	3	2
50M3	5	3	5	5	4	1	3	2	4	6	5	6	5	5	5	4	5	3	5	2	5	2	5	1	3	2	3	5	5	5	5	5
100M3	7	7	7	7	7	7	7	7	6	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7
Kendall's coefficient W (Eq. 5) = 0.6527																																
Spearman's $\rho_{s,j}$ (Eq. 12)																																
$\rho_{s,j}$	0.9	0.8	1.0	1.0	0.8	0.6	0.3	0.3	0.2	0.9	0.9	0.7	1.0	0.9	0.9	0.8	1.0	0.8	0.9	0.7	1.0	0.6	1.0	0.3	0.9	0.8	0.8	0.9	1.0	1.0	0.9	

334

335 **Table 9. Kendall's coefficient and Spearman's $\rho_{s,j}$ (superplasticiser variations)**

SCRC	τ_0 (15')	μ_{pl} (15')	τ_0 (45')	μ_{pl} (45')	τ_0 (90')	μ_{pl} (90')	$f_{c,3d}$	$f_{c,7d}$	$f_{c,28d}$	t500 (15')	SF (15')	tv (15')	PL (15')	t500j (15')	SFJ (15')	PJ (15')	SR	t500 (45')	SF (45')	tv (45')	PL (45')	t500j (45')	SFJ (45')	PJ (45')	t500 (90')	SF (90')	tv (90')	PL (90')	t500j (90')	SFJ (90')	PJ (90')	Rrb
	$R_{i,j}$																															
0	1	3	5	2	1	2	1	1	2	1	1	6	1	4	2	2	1	6	3	4	1	6	4	6	2	1	1	1	1	1	1	
20M1	2	2	2	4	2	5	7	3	6	3	5	7	3	3	5	6	3	4	2	3	2	2	3	5	4	4	2	2	3	2	3	3
50M1	5	4	3	3	3	3	4	6	5	2	3	3	4	5	4	5	2	3	1	2	3	4	6	3	5	5	3	5	4	5	4	4
100M1	4	5	4	5	6	6	2	2	3	6	6	2	6	6	6	3	6	5	5	1	6	5	5	4	6	6	6	6	6	6	6	6
20M3	3	1	1	1	4	4	5	4	4	4	2	4	5	2	1	4	4	1	4	5	4	1	1	1	3	3	4	4	2	3	2	2
50M3	6	6	6	6	5	1	6	7	7	5	4	1	2	1	3	1	5	2	6	6	5	3	2	2	1	2	5	3	5	4	5	5
100M3	7	7	7	7	7	7	3	5	1	7	7	5	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7	7
Kendall's coefficient W (Eq. 5) = 0.4026																																
Spearman's $\rho_{s,j}$ (Eq. 12)																																
$\rho_{s,j}$	0.9	0.9	0.6	0.9	0.9	0.5	0.0	0.4	-0.1	0.9	0.9	-0.5	0.7	0.5	0.8	0.3	0.9	0.3	0.6	0.1	0.9	0.4	0.6	0.2	0.6	0.8	0.9	0.8	1.0	0.9	1.0	

336

337

338

Table 10. Kendall’s coefficient and Spearman’s $\rho_{s,j}$ (cement variations)

SCRC	τ_0 (15')	μ_{pl} (15')	τ_0 (45')	μ_{pl} (45')	τ_0 (90')	μ_{pl} (90')	$f_{c,3d}$	$f_{c,7d}$	$f_{c,28d}$	t500 (15')	SF (15')	tv (15')	t500J (15')	SFJ (15')	PJ (15')	SR	t500 (45')	SF (45')	t500J (45')	SFJ (45')	PJ (45')	t500 (90')	SF (90')	t500J (90')	SFJ (90')	PJ (90')	Rrb
	$R_{i,j}$																										
0	2	3	1	4	1	1	2	2	1	1	1	1	4	4	1	2	1	1	4	4	4	1	2	1	2	1	1
20M1	1	1	3	2	2	2	1	1	2	3	2	4	3	3	3	2	1	2	3	2	3	3	1	3	1	2	2
50M1	3	2	4	3	3	3	3	4	4	2	3	3	1	2	2	3	3	3	1	1	1	2	3	2	3	3	3
100M1	4	4	2	1	4	4	4	3	3	4	4	2	2	1	1	4	4	4	2	3	2	4	4	4	4	4	4
Kendall’s coefficient W (Eq. 5) = 0.1402																											
Spearman’s $\rho_{s,j}$ (Eq. 12)																											
$\rho_{s,j}$	0.8	0.4	0.4	-0.8	1	1	0.8	0.6	0.8	0.8	1	0.2	-0.8	-1.0	-1.0	1	0.8	1	-0.8	-0.4	-0.8	0.8	0.8	0.8	0.8	1	1

339

340 To evaluate the significance of Kendall’s coefficient, a significance level (α) is chosen and then the
 341 critical value of W is determined (Table 11). When W equals or exceeds the critical value W^*
 342 obtained for a desired level of significance, it can be concluded that there is a good consensus
 343 among the properties used to evaluate robustness of the mixes.

344

Table 11. Critical values of Kendall’s coefficient (W^*)

α	W^*	
	$n = 7; k = 31$	$n = 4; k = 26$
0.05	0.0615	0.0880
0.01	0.0805	0.1229

345

346 In both water and superplasticiser variations, as W is greater than the critical value W^* , for any of
 347 the considered significance levels, it can be concluded with considerable confidence that there is
 348 agreement among the 31 properties ($k = 31$) concerning the evaluation of the robustness of the
 349 mixes.

350

351

352

In the case of cement variations, the W value calculated given 26 properties ($k = 26$) is slightly
 higher than the critical values for the $\alpha = 0.05$ and $\alpha = 0.01$ significance levels. Then, the selected
 properties to “judge” robustness will be also in agreement for the considered significance levels.

353 Once the significance of Kendall's coefficient has been evaluated, the correlation between the
354 rankings of an individual "judge" ($R_{i,j}$) and the final ranks of the objects, "Rrb", has to be assessed.
355 To do so, Spearman's correlation test is used, being it then necessary to obtain Spearman's rank
356 correlation coefficient.

357 In Table 8, Table 9 and Table 10, the Spearman's coefficient for each concrete property ($\rho_{s,j}$) is
358 calculated, Eq. 12, for water, superplasticiser and cement variations respectively.

359 A positive result of this Spearman's $\rho_{s,j}$ implies a good correlation between the evaluation (ranking)
360 obtained with this property and the final evaluation (rank) obtained in the mix when all studied
361 properties are considered. A negative $\rho_{s,j}$ value indicates non correlation between the evaluation
362 made with this property and the final evaluation obtained in the mix.

363 Therefore, those "judges" (properties) which provide no correlation have to be eliminated and
364 those that provide low correlation can also be removed to simplify the robustness (characteristic)
365 assessment. In this way, the number of properties ("judges") is changed and again, Kendall's
366 coefficient has to be calculated and its significance checked according to the desired level of
367 significance. Once this has been done, it can be concluded that the selection of properties that
368 provide the best correlation to assess the robustness is achieved.

369 Then, some of the 31 properties that exhibited negative or low $\rho_{s,j}$ values were removed to reduce
370 the number of properties that could be used for the evaluation of SCRC robustness. As a result, a
371 minimum of six properties were selected: two rheological properties, τ_0 (15') and μ_{pl} (15'), and four
372 workability parameters, t500 (15'), SF (15'), SFJ (15') and SR. This selection took into account the
373 $\rho_{s,j}$ values obtained in the three material variations (water, superplasticiser and cement) (Table 8,
374 Table 9 and Table 10). Moreover, these six properties would describe the rheological properties
375 (fundamental physical quantities) and the three key workability characteristics (empirical physical
376 quantities) of a SCRC mix.

377 The robustness categories determined using the six selected properties can be observed in Table
 378 12, Table 13 and Table 14 for water, superplasticiser and cement variations, respectively. Both sets
 379 of properties, the full 31 and the 6 selected properties, showed the same results regarding
 380 robustness evaluation of the seven SCRC mixes (in general terms of high, medium-high, medium-
 381 low and low).

382 **Table 12. Kendall's coefficient and Spearman's $\rho_{s,j}$ (6 properties - water variations)**

SCRC	τ_0 (15')	μ_{pl} (15')	t500 (15')	SF (15')	SFJ (15')	SR	Rrb	Robustness
	$R_{i,j}$							
0	1	1	1	1	2	1	1	High
20M1	2	2	2	3	4	2	2	Medium-high
50M1	3	5	4	6	3	4	4	Medium-low
100M1	6	6	5	4	6	6	6	Low
20M3	4	4	3	2	1	3	3	Medium-high
50M3	5	3	6	5	5	5	5	Medium-low
100M3	7	7	7	7	7	7	7	Low
Kendall's coefficient (W) (Eq. 5) = 0.8433								
Spearman's $\rho_{s,j}$ (Eq. 12)								
ρ_s	0.96	0.89	0.96	0.82	0.82	1.00		

383

384 **Table 13. Kendall's coefficient and Spearman's $\rho_{s,j}$ (6 properties - superplasticiser variations)**

SCRC	τ_0 (15')	μ_{pl} (15')	t500 (15')	SF (15')	SFJ (15')	SR	Rrb	Robustness
	$R_{i,j}$							
0	1	3	1	1	2	1	1	High
20M1	2	2	3	5	5	3	3	Medium-high
50M1	5	4	2	3	4	2	4	Medium-high
100M1	4	5	6	6	6	6	6	Low
20M3	3	1	4	2	1	4	2	Medium-high
50M3	6	6	5	4	3	5	5	Medium-low
100M3	7	7	7	7	7	7	7	Low
Kendall's coefficient (W) (Eq. 5) = 0.7619								
Spearman's $\rho_{s,j}$ (Eq. 12)								
ρ_s	0.86	0.86	0.86	0.89	0.82	0.86		

385

386

387

388

Table 14. Kendall's coefficient and Spearman's $\rho_{s,j}$ (6 properties - cement variations)

SCRC	τ_0 (15')	μ_{pl} (15')	t500 (15')	SF (15')	SFJ (15')	SR	Rrb	Robustness
	$R_{i,j}$							
0	2	3	1	1	4	1	1	High
20M1	1	1	3	2	3	2	2	High
50M1	3	2	2	3	2	3	3	Medium-high
100M1	4	4	4	4	1	4	4	Low
Kendall's coefficient (W) (Eq. 5) = 0.3000								
Spearman's $\rho_{s,j}$ (Eq. 12)								
ρ_s	0.80	0.40	0.80	1.00	-1	1		

389

390 Again, to determine the significance of W , a significance level (α) has to be chosen and the critical
 391 value of W for this α obtained (Table 15) [50]. If the calculated W (Table 12, Table 13 and Table 14)
 392 is greater than or equal to the critical value of the Kendall's coefficient W^* for any particular level
 393 of significance, Table 15, then there is a good agreement among the properties used to evaluate
 394 robustness.

395

Table 15. Critical values of Kendall's coefficient (W^*)

α	W^*	
	$n = 7, k = 6$	$n = 4, k = 6$
0.05	0.2589	0.3276
0.01	0.3351	0.4505

396

397 As it can be seen, in both water and superplasticiser variations, W exceeds the critical value W^* for
 398 all the considered significance levels. So, it can be concluded with considerable confidence that
 399 there is a high agreement among the selected 6 properties ($k = 6$) when water or superplasticiser
 400 vary.

401 The $\rho_{s,j}$ values were recalculated with the final ranking (Rrb) obtained for each mix (according to
 402 the sum of rankings obtained with the six selected properties). They are presented in Table 12,
 403 Table 13 and Table 14. According to these $\rho_{s,j}$, it can be concluded that τ_0 (15 min), μ_{pl} (15 min),
 404 t500 (15 min), SF (15 min), SFJ (15 min) and SR can be successfully used to assess the SCRC
 405 robustness due to the fact that all of them suitably correlate with the final result obtained.

406 In the case of cement variations, the W value calculated with the six selected properties was lower
407 than the critical value W^* for both $\alpha = 0.05$ and $\alpha = 0.01$ significance levels. As seen when 26
408 properties were considered, cement variations are less sensitive to evaluate robustness than
409 water and superplasticiser ones (it would be necessary to make more tests to evaluate the SCRC
410 robustness).

411 Lastly, it can be seen that when water variations are imposed the values of Kendall's coefficient
412 and Spearman's coefficient are the highest ones. Therefore, according to the results of this
413 statistical approach, introducing water variations in the mix is the most effective procedure to
414 asses SCRC robustness.

415 Comparing these results with those obtained by Naji et al. [21] for conventional self-compacting
416 concrete, it is observed that also in SCC variations in sand humidity and consequently water
417 variations should be controlled to ensure concrete behaviour. Moreover, in both cases, recycled
418 and conventional self-compacting concrete, static yield stress and plastic viscosity using a
419 rheometer are key properties to control self-compacting robustness. It means that rheology is a
420 robust tool to characterize any type of concrete in its fresh state and as a fluid. In addition, it
421 would be interesting to use a couple of empirical characterization tests to check filling ability,
422 passing ability and segregation resistance. In agreement with Naji et al. [21], the obtained results
423 suggest the use of J-Ring test and in this work, according to the analysis developed, the slump flow
424 test is really recommended. For the segregation resistance, both the surface settlement (proposed
425 by Naji et al. [21]), or the sieve segregation test, used in this work, can be accurately employed.
426 Finally, on the contrary to Naji et al. [21], the results suggest that compressive strength is not a key
427 property to evaluate robustness.

428 4 CONCLUSIONS

429 The robustness of self-compacting recycled concrete (SCRC) was deeply analysed. Based on the
430 results obtained, the following conclusions can be drawn:

431 The key materials that have to be controlled when SCRC robustness is taken into account in an
432 industrial production are the recycled aggregate percentage and the water variations (especially
433 those due to aggregate moisture). When low replacement percentages of recycled coarse
434 aggregate are used, SCRC shows a higher level of robustness. Moreover, when aggregates are used
435 with a moisture content, the control of water is more difficult and this affects SCRC robustness
436 negatively. Therefore, in a real production process, previous moisture of recycled aggregate has to
437 be thoroughly controlled.

438 In general, the 20% replacement concretes show a medium-high level of robustness and SCRCs
439 with a 100% of recycled aggregate display low robustness. Regarding the 50% replacement
440 concretes, the level of robustness depends largely on the mixing procedure in terms of water
441 control and previous moisture of recycled aggregates.

442 Moreover, the statistical approach based on Kendall's coefficient of concordance and Spearman's
443 rank correlation was successfully used to identify six key properties of SCRC that can be measured
444 to evaluate robustness: τ_0 (15 min), μ_{pl} (15 min), t_{500} (15 min), SF (15 min), SFJ (15 min) and SR.
445 These parameters are practically the same as those suggested in the literature [21] to evaluate
446 conventional self-compacting concrete.

447 Finally, according to this statistical approach, and in agreement with other studies developed with
448 conventional self-compacting concrete, water variation is the key factor that affects SCRC
449 robustness. In fact, in this work it has been observed that this type of concrete is more sensitive to
450 water variations than conventional SCC. Therefore, introducing water variations in the mix is the
451 most effective procedure to assess SCRC robustness.

452

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453

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455

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457

Appendix

458

Table 16. Test results and ranking of SCRCs according to COV of properties at different water levels

		SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3		
τ_0 (15')	W-; 0; W+	93.2	79.0	56.3	114	90.0	65.3	145	114	72.3	245	132	82.0	152	107	61.1	204	147	70.6	712	136	140
	COV (%)	24.5			27.3			32.9			54.4			42.6			47.7			101		
	Rank	1			2			3			6			4			5			7		
μ_{pl} (15')	W-; 0; W+	38.7	30.8	28.7	39.3	31.8	28.7	48.4	33.0	31.0	83.9	57.9	32.4	47.5	34.5	29.1	53.3	45.7	34.8	180	52.4	51.2
	COV (%)	16.2			16.3			25.8			44.3			25.5			20.8			78.4		
	Rank	1			2			5			6			4			3			7		
τ_0 (45')	W-; 0; W+	238	214	179	326	251	228	395	297	237	776	361	238	335	262	246	533	309	266	1607	328	449
	COV (%)	14.0			19.1			25.8			61.4			16.9			38.8			88.8		
	Rank	1			3			4			6			2			5			7		
μ_{pl} (45')	W-; 0; W+	43.7	32.8	31.6	45.6	33.0	32.8	56.9	36.5	33.1	129	63.7	41.4	49.8	38.7	32.4	84.3	50	40.8	225	60.7	77.4
	COV (%)	18.6			19.7			30.6			58.4			21.8			39.3			74.7		
	Rank	1			2			4			6			3			5			7		
τ_0 (90')	W-; 0; W+	742	515	482	898	644	523	917	846	530		1079	804	1397	587	474	2714	1076	1077		1119	3053
	COV (%)	24.4			27.8			26.9						61.5			58.3					
	Rank	1			3			2			6			5			4			7		
μ_{pl} (90')	W-; 0; W+	60.5	35.0	34.4	79.8	43.0	40.6	116	58.3	48.2		139	88.8	109	54.0	47.1	123	92.4	65.7		140	257
	COV (%)	34.4			40.3			49.6						48.4			30.6					
	Rank	2			3			5			6			4			1			7		
$f_{c,3d}$	W-; 0; W+	68.6	68.3	67.2	66.5	64.2	64.8	64.5	64.2	63.8	60.6	59.9	59.5	66.9	66.8	66.3	64.9	64.8	63.9	63.1	60.0	59.1
	COV (%)	1.1			1.8			0.5			1.0			0.4			0.8			3.4		
	Rank	5			6			2			4			1			3			7		
$f_{c,7d}$	W-; 0; W+	74.9	73.8	73.2	74.4	70.2	70.2	68.1	68.1	67.9	66.6	64.2	64.9	71.4	70.9	70.7	69.2	69.5	69.3	67.5	65.3	61.6
	COV (%)	1.2			3.4			0.2			1.9			0.5			0.3			4.6		
	Rank	4			6			1			5			3			2			7		
$f_{c,28d}$	W-; 0; W+	80.8	80.4	79.6	80.5	76.9	75.5	76.3	75.5	73.6	70.4	70.5	70.0	80.8	79.0	79.0	76.1	75.9	74.2	72.0	69.3	69.3
	COV (%)	0.8			3.3			1.9			0.4			1.3			1.4			2.2		
	Rank	2			7			5			1			3			4			6		
t_{500} (15')	W-; 0; W+	1.59	1.45	1.1	2.26	1.96	1.34	2.57	2.38	1.51	5.45	4.07	2.95	2.4	2.29	1.43	3.81	2.59	1.7		4.41	3.14
	COV (%)	18.3			25.3			26.2			30.1			26.0			39.2					
	Rank	1			2			4			5			3			6			7		
SF	W-; 0; W+	770	815	850	745	745	820	700	710	815	630	680	720	710	715	780	640	705	750		660	650

		SCRCO			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3		
(15')	COV (%)	4.9			5.6			8.6			6.7			5.3			7.9					
	Rank	1			3			6			4			2			5			7		
tv (15')	W-; 0; W+	29.5	23.7	18.4	39.0	25.8	25.7	40.6	30.6	24.9	43.1	33.2	26.4	34.0	24.8	23.9	47.3	32.5	27.6		22.0	14.6
	COV (%)	23.3			25.5			24.8			24.5			20.3			28.5					
	Rank	2			5			4			3			1			6			7		
PL (15')	W-; 0; W+	0.85	0.90	0.90	0.82	0.87	0.90	0.74	0.88	0.90	0.57	0.83	0.89	0.84	0.86	0.92	0.67	0.82	0.91		0.79	0.76
	COV (%)	3.3			4.7			10.4			22.3			4.8			15.2					
	Rank	1			2			4			6			3			5			7		
t500J (15')	W-; 0; W+	3.00	2.5	1.60	3.03	2.96	1.76	4.46	3.73	2.37	9.64	4.25	2.64	3.77	3.22	2.33	5.07	3.91	2.40		4.50	3.96
	COV (%)	29.9			27.6			30.1			66.5			23.4			35.3					
	Rank	3			2			4			6			1			5			7		
SFJ (15')	W-; 0; W+	750	820	850	730	750	845	670	700	775	535	675	745	695	725	735	620	690	730		660	665
	COV (%)	6.3			7.9			7.6			16.5			2.9			8.2					
	Rank	2			4			3			6			1			5			7		
PJ (15')	W-; 0; W+	12	10	9	18	13	8	23	19	16	31	20	18	16	14	12	23	17	13		20	20
	COV (%)	14.8			38.5			18.2			30.4			14.3			28.5					
	Rank	2			6			3			5			1			4			7		
SR	W-; 0; W+	11.1	13.6	15.3	8.9	13.1	13.5	7.5	11.5	13.4	2.7	3.5	7.6	7.3	10.6	12.9	5.6	9.4	11.8	0.02	4.8	2.0
	COV (%)	15.7			21.4			27.9			57.5			27.9			34.9			105		
	Rank	1			2			4			6			3			5			7		
t500 (45')	W-; 0; W+	2.39	1.95	1.9	3.3	2.31	2.21	3.53	2.75	2.57	8.77	5.41	3.53	3	2.58	2.48	4.13	3.46	3.01		5.71	2.95
	COV (%)	12.9			23.1			17.3			44.9			10.3			15.9					
	Rank	2			5			4			6			1			3			7		
SF (45')	W-; 0; W+	770	800	800	695	740	785	690	705	755	500	630	675	670	715	750	620	700	725		620	610
	COV (%)	2.2			5.9			4.6			15.1			5.6			8.0					
	Rank	1			4			2			6			3			5			7		
tv (45')	W-; 0; W+	33.3	24.7	21.2	45.5	35.2	22.5	59.3	45.3	33.0		42.1	40.2	34.9	28.1	26.6	43.9	34.1	31.5		32.9	21.3
	COV (%)	23.6			33.5			28.7						14.9			17.9					
	Rank	3			5			4			6			1			2			7		
PL (45')	W-; 0; W+	0.83	0.90	0.90	0.81	0.82	0.89	0.75	0.82	0.87	0.38	0.84	0.90	0.85	0.86	0.90	0.68	0.83	0.92		0.80	0.73
	COV (%)	4.6			5.2			7.4			40.3			3.0			15.3					
	Rank	2			3			4			6			1			5			7		
t500J (45')	W-; 0; W+	3.37	2.47	1.75	4.20	3.17	2.38	5.01	4.59	2.43		6.00	4.21	4.63	3.49	2.82	6.09	5.08	3.50		6.59	9.65
	COV (%)	32.1			28.1			34.5						25.1			26.7					
	Rank	4			3			5			6			1			2			7		
SFJ (45')	W-; 0; W+	740	790	795	700	745	760	650	690	750		630	700	660	725	760	600	680	720		620	525
	COV (%)	3.9			4.2			7.2						7.1			9.0					
	Rank	1			2			4			6			3			5			7		
PJ (45')	W-; 0; W+	15	10	10	21	15	15	25	23	17		26	25	30	20	15	26	21	20		24	40
	COV (%)	24.7			20.4			19.2						35.3			14.4					
	Rank	4			3			2			6			5			1			7		
t500 (90')	W-; 0; W+	4.71	2.44	2.13	8.28	2.8	2.58	13.0	5.83	3.53			5.69	8.52	4.44	2.9	12.9	7	3.95			
	COV (%)	45.5			70.9			66.3						54.9			57.4					
	Rank	1			5			4			6			2			3			7		
SF (90')	W-; 0; W+	705	715	785	570	690	730	495	640	705		455	565	510	660	700	490	570	620		435	
	COV (%)	5.9			12.5			17.5						16.1			11.7					
	Rank	1			3			5			6			4			2			7		
tv	W-; 0; W+	47.2	34.5	28.5	73.0	48.8	29.1		61.8	54.7			60		65	36.2		70.3	64.2			

		SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3		
(90')	COV (%)	26.0			43.7																	
	Rank	1			2			4			6			5			3			7		
PL (90')	W-; 0; W+	0.54	0.75	0.82	0.38	0.60	0.91	0.38	0.62	0.77		0.56	0.51	0.51	0.73	0.74	0.21	0.60	0.89		0.17	
	COV (%)	20.7			42.8			33.3						19.7			60.2					
t500J (90')	Rank	2			4			3			6			1			5			7		
	W-; 0; W+	5.42	3.12	2.96	7.82	4.83	3.50	22.7	7.44	6.22			13.1	11.6	7.69	4.12		12.4	8.12			
SFJ (90')	COV (%)	35.9			41.1			75.8						48.1								
	Rank	1			2			4			6			3			5			7		
PJ (90')	W-; 0; W+	690	720	750	610	660	700	475	590	650			525	510	600	690		530	570			
	COV (%)	4.2			6.9			15.6						15.0								
SR _i	Rank	1			2			4			6			3			5			7		
	W-; 0; W+	25	17	16	35	26	25	59	35	32			50	49	35	25		44	35			
Rrb	COV (%)	26.2			19.7			35.2						33.2								
	Rank	2			1			4			6			3			5			7		
		57			104			116			171			80			124			216		
		1			3			4			6			2			5			7		

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Table 17. Test results and ranking of SCRCs according to COV of properties at different superplasticiser levels

		SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3		
τ ₀ (15')	S-; 0; S+	87.6	79.0	80.2	105	90.0	83.0	136	114	90.7	155	132	105	128	107	88.3	181	147	98.9	524	136	163
	COV (%)	5.7			12.0			20.0			19.2			18.5			28.9			79.0		
	Rank	1			2			5			4			3			6			7		
μ _{pl} (15')	S-; 0; S+	34.5	30.8	29.1	36.1	31.8	31.0	38.3	33.0	32.5	61.4	57.9	42.1	35.7	34.5	33.6	65.8	45.7	40.9	125	52.4	57.9
	COV (%)	8.7			8.3			9.3			19.1			3.1			26.0			51.8		
	Rank	3			2			4			5			1			6			7		
τ ₀ (45')	S-; 0; S+	264	214	201	265	251	244	316	297	263	392	361	308	265	262	264	522	309	287	1365	328	465
	COV (%)	14.6			4.3			9.3			12.0			0.7			34.8			78.3		
	Rank	5			2			3			4			1			6			7		
μ _{pl} (45')	S-; 0; S+	40.0	32.8	32.1	43.5	33.0	32.3	47.9	36.5	36.2	79.1	63.7	54.2	46.0	38.7	38.1	93.8	50.0	48.3	185	60.7	82.5
	COV (%)	12.5			17.3			16.6			19.1			10.8			40.3			60.7		
	Rank	2			4			3			5			1			6			7		
τ ₀ (90')	S-; 0; S+	556	515	463	816	644	508	1131	846	650	1787	1079	934	825	587	456	1600	1076	908		1119	3541
	COV (%)	9.1			23.5			27.6			36.0			30.1			30.2					
	Rank	1			2			3			6			4			5			7		
μ _{pl} (90')	S-; 0; S+	45.3	35.0	38.7	67.8	43.0	41.9	86.1	58.3	56.2	213	139	107	78.4	54.0	49.4	115	92.4	91.7		140	258
	COV (%)	13.1			28.8			25.0			35.7			25.7			13.1					
	Rank	2			5			3			6			4			1			7		
f _{c,3d}	S-; 0; S+	66.6	68.3	67.0	64.9	64.2	68.5	63.7	64.2	66.5	59.5	59.9	58.3	67.0	66.8	69.7	62.9	64.8	66.1	60.2	60.0	58.5
	COV (%)	1.3			3.4			2.3			1.4			2.3			2.5			1.6		
	Rank	1			7			4			2			5			6			3		
f _{c,7d}	S-; 0; S+	73.7	73.8	73.9	70.1	70.2	72.3	67.6	68.1	70.2	63.7	64.2	62.2	72.4	70.9	73.5	68.6	69.5	71.4	65.6	65.3	63.3
	COV (%)	0.2			1.7			2.0			1.6			1.8			2.03			1.9		
	Rank	1			3			6			2			4			7			5		

		SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3		
f _{c,28d}	S-; 0; S+	80.8	80.4	81.5	76.9	76.9	79.3	73.6	75.5	76.2	70.4	70.5	69.4	78.6	79.0	81.0	72.2	75.9	76.1	69.9	69.3	69.0
	COV (%)	0.73			1.8			1.75			0.9			1.6			2.9			0.7		
	Rank	2			6			5			3			4			7			1		
t500 (15')	S-; 0; S+	1.47	1.45	1.41	2.27	1.96	1.51	2.77	2.38	2.07	6.47	4.07	2.9	2.53	2.29	1.59	2.68	2.59	1.7		4.41	4
	COV (%)	2.1			20.0			14.6			40.6			22.9			23.3					
	Rank	1			3			2			6			4			5			7		
SF (15')	S-; 0; S+	790	815	820	720	745	780	695	710	730	568	680	700	695	715	725	670	705	700		660	620
	COV (%)	2.0			4.0			2.5			10.9			2.1			2.7					
	Rank	1			5			3			6			2			4			7		
tv (15')	S-; 0; S+	39.1	23.7	21.2	38.4	25.8	16.2	34.8	30.6	19.7	27.8	33.2	21.1	32.8	24.8	18.7	24.5	32.5	23.2	37.0	22.0	21.0
	COV (%)	34.6			41.5			27.4			22.1			27.7			18.9			33.7		
	Rank	6			7			3			2			4			1			5		
PL (15')	S-; 0; S+	0.89	0.90	0.91	0.84	0.87	0.89	0.81	0.88	0.84	0.69	0.83	0.83	0.82	0.86	0.90	0.79	0.82	0.83	0.24	0.79	0.82
	COV (%)	1.1			2.9			4.2			10.3			4.7			2.6			53.0		
	Rank	1			3			4			6			5			2			7		
t500J (15')	S-; 0; S+	3.44	2.5	1.90	3.84	2.96	2.15	4.88	3.73	2.32	10.4	4.25	4.00	3.38	3.22	2.90	4.18	3.91	3.62		4.50	5.07
	COV (%)	29.7			28.3			35.2			58.3			7.8			7.2					
	Rank	4			3			5			6			2			1			7		
SFJ (15')	S-; 0; S+	780	820	820	710	750	815	680	700	770	550	675	705	720	725	755	665	690	715		660	620
	COV (%)	2.9			7.0			6.6			12.8			2.6			3.6					
	Rank	2			5			4			6			1			3			7		
PJ (15')	S-; 0; S+	12	10	7	25	13	9	30	19	13	33	20	19	24	14	12	23	17	15		20	23
	COV (%)	26.0			53.1			41.7			31.6			37.5			23.4					
	Rank	2			6			5			3			4			1			7		
SR	S-; 0; S+	12.8	13.6	15.1	11.0	13.1	13.3	11.1	11.5	13.1	2.4	3.5	8.3	9.9	10.6	12.7	7.9	9.4	11.1	0.0	4.8	2.9
	COV (%)	8.5			9.8			9.2			66.6			13.2			16.5			94.5		
	Rank	1			3			2			6			4			5			7		
t500 (45')	S-; 0; S+	3.09	1.95	1.58	3.15	2.31	1.81	3.78	2.75	2.59	5.72	5.41	3.22	3.32	2.58	2.43	3.65	3.46	2.6		5.71	4.5
	COV (%)	35.7			27.9			21.2			28.5			17.2			17.3					
	Rank	6			4			3			5			1			2			7		
SF (45')	S-; 0; S+	745	800	810	715	740	765	690	705	725	585	630	680	690	715	795	660	700	790		620	610
	COV (%)	4.5			3.2			2.5			7.5			7.4			9.3					
	Rank	3			2			1			5			4			6			7		
tv (45')	S-; 0; S+	34.5	24.7	22.6	40.0	35.2	31.9	47.4	45.3	42.2	42.1	42.1	40.5	35.0	28.1	21.7	32.9	34.1	18.1		32.9	23.9
	COV (%)	23.2			11.4			5.8			2.1			23.6			31.3					
	Rank	4			3			2			1			5			6			7		
PL (45')	S-; 0; S+	0.89	0.90	0.91	0.80	0.82	0.83	0.80	0.82	0.85	0.69	0.84	0.85	0.81	0.86	0.86	0.78	0.83	0.87		0.80	0.69
	COV (%)	1.1			1.9			3.1			11.3			3.4			5.5					
	Rank	1			2			3			6			4			5			7		
t500J (45')	S-; 0; S+	3.46	2.47	1.94	3.63	3.17	2.76	5.51	4.59	3.26	8.27	6.00	5.03	3.71	3.49	3.13	6.00	5.08	4.01		6.59	6.21
	COV (%)	29.4			13.7			25.4			25.8			8.5			19.8					
	Rank	6			2			4			5			1			3			7		
SFJ (45')	S-; 0; S+	715	790	795	700	745	765	640	690	740	600	630	680	695	725	750	655	680	710		620	610
	COV (%)	5.8			4.5			7.2			6.3			3.8			4.2					
	Rank	4			3			6			5			1			2			7		
PJ (45')	S-; 0; S+	27	10	10	28	15	10	30	23	20	34	26	22	24	20	17	25	21	17		24	30
	COV (%)	62.6			52.6			21.1			22.4			17.6			19.0					
	Rank	6			5			3			4			1			2			7		

	SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			SCRC20M3			SCRC50M3			SCRC100M3			
t500 (90')	S-; 0; S+	3.71	2.44	1.93	5.12	2.8	2.36		5.83	4.13			9.05	6.42	4.44	2.78	8.93	7	4.44			
	COV (%)	34.0			43.3						40.1			33.2								
	Rank	2			4			5			6			3			1			7		
SF (90')	S-; 0; S+	695	715	760	560	690	700		640	645		455	485	550	660	695	515	570	585		435	
	COV (%)	4.4			12.0						11.9			6.9								
	Rank	1			4			5			6			3			2			7		
tv (90')	S-; 0; S+	41.5	34.5	34.1	66.6	48.8	42.0		61.8	52.0					65	36.1		70.3				
	COV (%)	11.3			24.2																	
	Rank	1			2			3			6			4			5			7		
PL (90')	S-; 0; S+	0.71	0.75	0.80	0.51	0.60	0.79		0.62	0.56		0.56	0.34	0.40	0.73	0.78	0.33	0.60	0.55		0.17	
	COV (%)	6.0			22.6									32.4			29.1					
	Rank	1			2			5			6			4			3			7		
t500J (90')	S-; 0; S+	4.66	3.12	2.88	8.00	4.83	3.76		7.44	5.57			18.8	10.9	7.69	5.76		12.4	5.87			
	COV (%)	27.2			39.9									32.3								
	Rank	1			3			4			6			2			5			7		
SFJ (90')	S-; 0; S+	670	720	730	575	660	680		590	675			510	510	600	690		530	605			
	COV (%)	4.5			8.7									15.0								
	Rank	1			2			5			6			3			4			7		
PJ (90')	S-; 0; S+	28	17	15	55	26	18		35	30			42	53	35	25		44	27			
	COV (%)	34.8			59.4									38.1								
	Rank	1			3			4			6			2			5			7		
SR_i	74			109			117			151			91			123			203			
Rrb	1			3			4			6			2			5			7			

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463 **Table 18. Test results and ranking of SCRCs according to COV of properties at different cement levels**

	SCRC0			SCRC20M1			SCRC50M1			SCRC100M1			
τ_0 (15')	C-; 0; C+	70.6	79.0	83.2	88.6	90.0	96.7	92.6	114	130	97.4	132	150
	COV (%)	8.3			4.7			16.7			20.8		
	Rank	2			1			3			4		
μ_{pl} (15')	C-; 0; C+	25.5	30.8	31.2	30.7	31.8	33.0	31.9	33.0	35.5	36.2	57.9	59.0
	COV (%)	10.9			3.6			5.5			25.2		
	Rank	3			1			2			4		
τ_0 (45')	C-; 0; C+	214	214	244	228	251	310	240	297	347	293	361	386
	COV (%)	7.7			16.2			18.2			13.8		
	Rank	1			3			4			2		
μ_{pl} (45')	C-; 0; C+	28.8	32.8	38.1	33.0	33.0	41.5	34.3	36.5	44.4	52.2	63.7	67.3
	COV (%)	14.0			13.7			13.8			12.9		
	Rank	4			2			3			1		
τ_0 (90')	C-; 0; C+	504	515	608	596	644	744	659	846	1164	879	1079	3967
	COV (%)	10.6			11.4			28.7			87.5		
	Rank	1			2			3			4		
μ_{pl} (90')	C-; 0; C+	34.5	35.0	45.0	40.8	43.0	68.8	56.8	58.3	104	87.7	139	198
	COV (%)	15.7			30.6			36.6			39.0		
	Rank	1			2			3			4		

		SCRCO			SCRC20M1			SCRC50M1			SCRC100M1		
f_{c,3d}	C-; 0; C+	66.2	68.3	69.8	63.7	64.2	65.1	60.5	64.2	60.8	55.1	59.9	56.8
	COV (%)	2.7			1.1			3.3			4.3		
	Rank	2			1			3			4		
f_{c,7d}	C-; 0; C+	71.4	73.8	75.7	70.9	70.2	71.1	62.2	68.1	67.6	59.3	64.2	62.0
	COV (%)	2.9			0.7			5.0			3.9		
	Rank	2			1			4			3		
f_{c,28d}	C-; 0; C+	79.8	80.4	80.6	76.7	76.9	78.5	69.5	75.5	73.8	63.9	70.5	67.0
	COV (%)	0.5			1.3			4.3			4.9		
	Rank	1			2			4			3		
t500 (15')	C-; 0; C+	1.39	1.45	1.78	1.57	1.96	2.93	1.97	2.38	3.35	2.21	4.07	4.21
	COV (%)	13.6			32.5			27.6			31.9		
	Rank	1			3			2			4		
SF (15')	C-; 0; C+	820	815	760	800	745	705	790	710	685	760	680	660
	COV (%)	4.2			6.4			7.5			7.6		
	Rank	1			2			3			4		
tv (15')	C-; 0; C+	22.9	23.7	25.8	23.9	25.8	38.8	25.1	30.6	37.7	27.7	33.2	22.6
	COV (%)	6.2			27.6			20.4			19.0		
	Rank	1			4			3			2		
t500J (15')	C-; 0; C+	1.71	2.5	2.88	2.19	2.96	3.38	3.15	3.73	3.87	3.31	4.25	4.70
	COV (%)	25.3			21.2			10.7			17.3		
	Rank	4			3			1			2		
SFJ (15')	C-; 0; C+	820	820	740	775	750	720	740	700	695	715	675	680
	COV (%)	5.8			3.5			3.5			3.2		
	Rank	4			3			2			1		
PJ (15')	C-; 0; C+	9	10	19	12	13	20	15	19	21	16	20	22
	COV (%)	42.7			29.1			16.7			15.8		
	Rank	4			3			2			1		
SR	C-; 0; C+	16.0	13.6	12.1	16.4	13.1	11.3	13.1	11.5	8.8	8.2	3.5	3.9
	COV (%)	14.2			18.9			19.8			50.4		
	Rank	1			2			3			4		
t500 (45')	C-; 0; C+	1.71	1.95	2.23	2.33	2.31	2.52	2.47	2.75	3.77	2.62	5.41	5.75
	COV (%)	13.3			4.9			22.8			37.4		
	Rank	2			1			3			4		
SF (45')	C-; 0; C+	780	800	750	740	740	695	735	705	665	723	630	600
	COV (%)	3.2			3.6			5.0			9.9		
	Rank	1			2			3			4		
t500J (45')	C-; 0; C+	2.11	2.47	2.92	2.70	3.17	3.70	4.50	4.59	4.61	4.90	6.00	6.13
	COV (%)	16.2			15.7			1.3			11.9		
	Rank	4			3			1			2		
SFJ (45')	C-; 0; C+	800	790	730	755	745	715	695	690	685	675	630	640
	COV (%)	4.9			2.7			0.7			3.6		
	Rank	4			2			1			3		
PJ (45')	C-; 0; C+	10	10	20	13	15	21	19	23	24	20	26	27
	COV (%)	42.5			24.8			12.7			14.9		
	Rank	4			3			1			2		
t500 (90')	C-; 0; C+	2.32	2.44	2.99	2.64	2.8	4.95	3.18	5.83	6.03	5		
	COV (%)	13.8			37.2			31.7					
	Rank	1			3			2			4		

		SCRC0			SCRC20M1			SCRC50M1			SCRC100M1		
SF (90')	C-; 0; C+	730	715	680	690	690	650	680	640	570	580	455	
	COV (%)	3.6			3.4			8.8					
	Rank	2			1			3			4		
t500J (90')	C-; 0; C+	2.91	3.12	3.57	3.82	4.83	5.75	6.08	7.44	8.50	14.9		
	COV (%)	10.5			20.1			16.5					
	Rank	1			3			2			4		
SFJ (90')	C-; 0; C+	750	720	705	660	660	650	640	590	555	525		
	COV (%)	3.0			0.9			7.2					
	Rank	2			1			3			4		
PJ (90')	C-; 0; C+	16	17	20	20	26	28	33	35	46	38		
	COV (%)	12.3			17.2			18.4					
	Rank	1			2			3			4		
	SR_i	55			56			67			82		
	Rrb	1			2			3			4		

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